

**DYNAMIC BUCKLING LOAD OF AN IMPERFECT VISCOUSLY DAMPED
SPHERICAL CAP STRESSED BY A STEP LOAD**

ABSTRACT

This paper determines the dynamic buckling load of a lightly and viscously damped imperfect spherical cap with a step load. The spherical cap is discretized into a pre-buckling symmetric mode and a buckling mode that consists of axisymmetric and non-axisymmetric buckling modes. The imperfection is taken at the shape of the buckling mode. The inherent problem contains a small parameter which necessitated the adoption of regular perturbation procedures, using asymptotic technique. The general result is designed to display the contributions of each of the terms in the governing differential equations. We deduce the results for the respective special cases where the axisymmetric imperfection parameter, namely $\bar{\xi}_1$, and the non-axisymmetric imperfection parameter $\bar{\xi}_2$, are zeros. We also determine the effects of each of the non-linear terms as well as the effects of the coupling term.

Keywords: Spherical cap, step load, dynamic buckling, imperfection parameter.

1. INTRODUCTION.

The subject of dynamic buckling of elastic structures has been a thriving area of investigation ever since [1-3] developed the discipline of dynamic stability of elastic structures from the original static consideration that was prevalent before this time. Over the years, many investigations on dynamic stability of elastic structures have been added to the original sketchy and scattered pieces that saw the genesis of dynamic buckling of elastic structures as a research interest. Among the many scholarly investigations that have come to light include [4], [5], [6], [7] and [8], who investigated the dynamic buckling, of two-degree – of freedom systems with mode interaction under step loading. Mention must also be made of relatively recent investigations which include [9], who investigated the dynamic buckling of thin cylindrical shells under axial impact, [10-11], who studied the nonlinear dynamic buckling of stiffened plates under in-plane impact load.

But by far, the investigation that concerns us in this study is that by [12], who investigated the dynamic buckling loads of imperfection-sensitive structures from perturbation

25 procedures. His analysis was predicated primarily on the studies earlier enunciated by [1-3].
26 Other Pertinent investigations include those by [13], [14] and [15, 16], among others.

27

28 However, a cursory appraisal of all the investigations to date reveals that the phenomenon of
29 damping has been given very little or no attention at all in the dynamic buckling process. We
30 are of the strong opinion that since dynamic buckling process is a time dependent process,
31 the effect of damping, no matter how slight, should not be overlooked. In this investigation,
32 the presence of a small viscous damping is therefore assumed and given some level of
33 prominence. Of course, the result obtained is far more representative of the actual physical
34 life situation. To this end, we remark that a few of the many existing investigations that have
35 tended to incorporate damping include the studies by [17-20], among others.

36

37 The layout of this investigation is as follows:

38 We shall first write down the mathematical equations satisfied by the structure investigated.

39

40 We shall next develop asymptotic techniques, using perturbation procedures to solve the
41 governing equations analytically. We note that dynamic bucking problems are always non
42 linear and therefore, closed-form exact solutions are not always possible. Therefore, regular
43 perturbation method provides a suitable alternative to the solution of such problems,
44 particularly when the problems contain small parameters in which asymptotic series
45 expansion can always be invoked.

46

47 We shall lastly make pertinent deductions.

48

49

50 There are five sections in this paper. Section two examines the dynamic buckling load of an
 51 imperfect viscously damped spherical cap stressed by a step load. Section three introduces
 52 the viscous damping to Danielson's results. Section four considers the analysis of results
 53 while section five ends this work with a conclusion.

54

55 **2. THE DYNAMIC BUCKLING LOAD.**

56

57 Danielson, had, for simplicity, assumed that the normal displacement $w(x,y,T)$ of the
 58 spherical cap was given as

$$59 \quad W(x,y,T) = \xi_0(T)W_0(x,y) + \xi_1(T)W_1(x,y) + \xi_2(T)W_2(x,y) \quad (1)$$

60 where $W_0(x,y)$ is the pre-buckling mode and $W_1(x,y), W_2(x,y)$ are the axisymmetric and
 61 non-axisymmetric modes respectively. $\xi_0(T), \xi_1(T)$ and $\xi_2(T)$ are the respective time

62 dependent amplitudes of the associated modes. Imperfection \bar{W} was introduced as

$$63 \quad \bar{W} = \bar{\xi}_1 W_1 + \bar{\xi}_2 W_2 \quad (2)$$

64 where W_1, W_2 still have meanings as before and $\bar{\xi}_1, \bar{\xi}_2$ are the imperfect amplitudes
 65 assumed to be small relative to unity. On assuming suitable forms for W_0, W_1, W_2 and
 66 substituting same into the compatibility and dynamic equilibrium equations and simplifying,
 67 using his assumptions, Danielson obtained the following coupled differential equations for
 68 step loading

$$69 \quad \frac{1}{\omega_0^2} \frac{d^2 \xi_0}{dT^2} + \xi_0 = \lambda f(T) \quad (3)$$

$$70 \quad \frac{1}{\omega_1^2} \frac{d^2 \xi_1}{dT^2} + \xi_1(1 - \xi_0) - k_1 \xi_1^2 + k_2 \xi_2^2 = \bar{\xi}_1 \xi_0 \quad (4)$$

$$71 \quad \frac{1}{\omega_2^2} \frac{d^2 \xi_2}{dT^2} + \xi_2(1 - \xi_0) + \xi_1 \xi_2 = \bar{\xi}_2 \xi_0 \quad (5)$$

$$72 \quad \xi_i(0) = \xi_i'(0) = 0; i = 1, 2.$$

73 Here, $f(T)$ is the loading history which in our investigation, (as in Danielson's case), is the
 74 step load characterized by

$$f(T) = \begin{cases} 1, & T > 0 \\ 0, & T < 0 \end{cases}, \quad (6)$$

and, λ , is the load parameter, considered to be non-dimensionalized and satisfies the inequality $0 < \lambda < 1$.

In our quest for solution, we are to determine a particular value of λ , called the dynamic buckling load represented by λ_D and which satisfies the inequality $0 < \lambda_D < 1$. We define the dynamic buckling load λ_D as the largest load parameter such that the solution to the damped version of problems (3)-(6) remains bounded for all time $T > 0$. As in (3)-(5), we note that $\omega_i; i = 0, 1, 2$ are the circular frequencies of the associated modes ξ_0, ξ_1 and ξ_2 respectively while k_1 and k_2 are constants considered positive

84

85 **3. THE USE OF VISCOUS DAMPING IN DANIELSON'S RESULTS.**

86

The present study is an extension of Danielson's problem to the case where a small viscous damping is present. We however avoid Danielson's method (who used Mathieu – type of instability), for, as noted by [3, page 100], Mathieu – type of instability is always associated with many cycles of oscillations as opposed to just one shot of oscillation that triggers off dynamic buckling.

For simplicity of analysis, we assume the existence of damping on the buckling modes.

Since this damping must be only proportional to the velocity, we add the terms $c_1 \frac{d\xi_1}{dT}$ and

$c_2 \frac{d\xi_2}{dT}$ to (4) and (5) respectively and the formulation now becomes

$$\frac{1}{\omega_0^2} \frac{d^2 \xi_0}{dT^2} + \xi_0 = \lambda f(T) \quad (7)$$

$$\frac{1}{\omega_1^2} \frac{d^2 \xi_1}{dT^2} + c_1 \frac{d\xi_1}{dT} + \xi_1(1 - \xi_0) - k_1 \xi_1^2 + k_2 \xi_2^2 = \bar{\xi}_1 \xi_0 \quad (8)$$

$$\frac{1}{\omega_2^2} \frac{d^2 \xi_2}{dT^2} + c_2 \frac{d\xi_2}{dT} + \xi_2(1 - \xi_0) + \xi_1 \xi_2 = \bar{\xi}_2 \xi_0 \quad (9)$$

where $c_i, i = 1, 2$ are the damping constants and which satisfy the inequality $0 < c_i < 1$.

Using $f(T) = 1$ and substituting (6) into (7) we have

$$\frac{1}{\omega_0^2} \frac{d^2 \xi_0}{dT^2} + \xi_0 = \lambda \quad (10)$$

$$\frac{1}{\omega_1^2} \frac{d^2 \xi_1}{dT^2} + c_1 \frac{d\xi_1}{dT} + \xi_1(1 - \xi_0) - k_1 \xi_1^2 + k_2 \xi_2^2 = \bar{\xi}_1 \xi_0 \quad (11)$$

$$\frac{1}{\omega_2^2} \frac{d^2 \xi_2}{dT^2} + c_2 \frac{d\xi_2}{dT} + \xi_2(1 - \xi_0) + \xi_1 \xi_2 = \bar{\xi}_2 \xi_0 \quad (12)$$

Now using,

$$t = \omega_0 T,$$

so that

$$\frac{d(\quad)}{dT} = \omega_0 \frac{d(\quad)}{dt}, \quad \frac{d^2(\quad)}{dT^2} = \omega_0^2 \frac{d^2(\quad)}{dt^2},$$

Then (10)–(12) become

$$\frac{d^2 \xi_0}{dt^2} + \xi_0 = \lambda \quad (13)$$

109

$$\frac{d^2 \xi_1}{dt^2} + \left[\frac{c_1 \omega_0 \omega_1^2}{\omega_0^2} \right] \frac{d\xi_1}{dt} + \left[\frac{\omega_1}{\omega_0} \right]^2 \xi_1(1 - \xi_0) - \left[\frac{\omega_1}{\omega_0} \right]^2 k_1 \xi_1^2 + \left[\frac{\omega_1}{\omega_0} \right]^2 k_2 \xi_2^2 = \left[\frac{\omega_1}{\omega_0} \right]^2 \bar{\xi}_1 \xi_0 \quad (14)$$

$$\frac{d^2 \xi_2}{dt^2} + \left[\frac{c_2 \omega_0 \omega_2^2}{\omega_0^2} \right] \frac{d\xi_2}{dt} + \left[\frac{\omega_2}{\omega_0} \right]^2 \xi_2(1 - \xi_0) + \left[\frac{\omega_2}{\omega_0} \right]^2 \xi_1 \xi_2 = \left[\frac{\omega_2}{\omega_0} \right]^2 \bar{\xi}_2 \xi_0 \quad (15)$$

Next, we let

$$2\alpha_1 \varepsilon = \frac{c_1 \omega_1^2}{\omega_0}, 2\alpha_2 \varepsilon = \frac{c_2 \omega_2^2}{\omega_0}, Q = \frac{\omega_1}{\omega_0}, R = \frac{\omega_2}{\omega_0}, S = \left[\frac{\omega_2}{\omega_1} \right]^2 \quad (16)$$

where,

$$\varepsilon = \lambda Q^2 = \lambda \left[\frac{\omega_1}{\omega_0} \right]^2, \quad (17)$$

and

$$0 < \alpha_1 < 1, \quad 0 < \alpha_2 < 1, \quad 0 < Q < 1, \quad 0 < R < 1 \text{ and } 0 < \varepsilon < 1$$

Substituting (16) into (14) and (15) yield

$$\frac{d^2 \xi_0}{dt^2} + \xi_0 = \lambda \quad (18)$$

$$\frac{d^2 \xi_1}{dt^2} + 2\alpha_1 \varepsilon \frac{d\xi_1}{dt} + Q^2 \xi_1 (1 - \xi_0) - k_1 Q^2 \xi_1^2 + k_2 Q^2 \xi_2^2 = Q^2 \bar{\xi}_1 \xi_0 \quad (19)$$

$$\frac{d^2 \xi_2}{dt^2} + 2\alpha_2 \varepsilon \frac{d\xi_2}{dt} + R^2 \xi_2 (1 - \xi_0) + R^2 \xi_1 \xi_2 = R^2 \bar{\xi}_2 \xi_0 \quad (20)$$

$$\xi_i(0) = \xi'_i(0) = 0; i = 1, 2.$$

As in [1–3], we neglect the pre-buckling inertia term, so that from (18) we get

$$\xi_0 = \lambda \quad (21)$$

On simplification, using (21), equations (19) and (20) yield

$$\frac{d^2 \xi_1}{dt^2} + 2\alpha_1 \varepsilon \frac{d\xi_1}{dt} + Q^2 \xi_1 - \varepsilon \xi_1 - k_1 Q^2 \xi_1^2 + k_2 Q^2 \xi_2^2 = \varepsilon \bar{\xi}_1 \quad (22)$$

and

$$\frac{d^2 \xi_2}{dt^2} + 2\alpha_2 \varepsilon \frac{d\xi_2}{dt} + R^2 \xi_2 - \varepsilon S \xi_2 + R^2 \xi_1 \xi_2 = \varepsilon S \bar{\xi}_2 \quad (23)$$

$$\xi_i(0) = \xi'_i(0) = 0; i = 1, 2$$

where,

$$S = \left[\frac{R}{Q} \right]^2.$$

We assume a small time scale τ such that,

$$\tau = \varepsilon t \quad (24a)$$

and

$$\xi'_i = \xi_{i,t} + \varepsilon \xi_{i,\tau} \quad (24b)$$

$$\xi''_i = \xi_{i,tt} + 2\varepsilon \xi_{i,t\tau} + \varepsilon^2 \xi_{i,\tau\tau}; i = 1, 2 \quad (24c)$$

We denote our perturbation parameter by ε so that

$$\xi_1(t) = \sum_{i=1}^{\infty} \zeta^{(i)}(t, \tau) \varepsilon^i \quad (25)$$

$$\xi_2(t) = \sum_{i=1}^{\infty} \eta^{(i)}(t, \tau) \varepsilon^i \quad (26)$$

Substituting (25) and (26) into (22) and (23), using (24b) and (24c), equating terms of the orders of ε we get,

$$\zeta_{,tt}^{(1)} + Q^2 \zeta^{(1)} = \bar{\xi}_1 \quad (27)$$

$$\zeta_{,tt}^{(2)} + Q^2 \zeta^{(2)} = -2\alpha_1 \zeta_{,t}^{(1)} + \zeta^{(1)} + k_1 Q^2 \zeta^{(1)^2} - k_2 Q^2 \eta^{(1)^2} - 2\zeta_{,t\tau}^{(1)} \quad (28)$$

and

$$\eta_{,tt}^{(1)} + R^2 \eta^{(1)} = S \bar{\xi}_2 \quad (29)$$

$$\eta_{,tt}^{(2)} + R^2 \eta^{(2)} = -2\alpha_2 \eta_{,t}^{(1)} + S \eta^{(1)} - 2\eta_{,t\tau}^{(1)} - R^2 \zeta^{(1)} \eta^{(1)} \quad (30)$$

$$\zeta^{(i)}(0,0) = \eta^{(i)}(0,0) = 0, i = 1, 2 \quad (31)$$

$$\zeta_{,t}^{(i)}(0,0) = \eta_{,t}^{(i)}(0,0) = 0, i = 1, 2 \quad (32)$$

$$\zeta_{,t}^{(i+1)}(0,0) + \zeta_{,\tau}^{(i)}(0,0) = \eta_{,t}^{(i+1)}(0,0) + \eta_{,\tau}^{(i)}(0,0) = 0, i = 1, 2 \quad (33)$$

The solution of (27) using (31) and (32) is

$$\zeta^{(1)}(t, \tau) = a_1(\tau) \cos Qt + b_1(\tau) \sin Qt + \frac{\bar{\xi}_1}{Q^2} \quad (34a)$$

$$a_1(0) = -\frac{\bar{\xi}_1}{Q^2}; b_1(0) = 0 \quad (34b)$$

Similarly, the solution of (28) is

$$\eta^{(1)}(t, \tau) = a_2(\tau) \cos Rt + b_2(\tau) \sin Rt + \frac{S \bar{\xi}_2}{R^2} \quad (35a)$$

$$a_2(0) = -\frac{S \bar{\xi}_2}{R^2}; b_2(0) = 0 \quad (35b)$$

Substituting using (34a) and (35a) into (28), we have

$$\begin{aligned} \zeta_{,tt}^{(2)} + Q^2 \zeta^{(2)} = & -2\alpha_1 [-Qa_1 \sin Qt + Qb_1 \cos Qt] + \left[a_1 \cos Qt + b_1 \sin Qt + \frac{\bar{\xi}_1}{Q^2} \right] \\ & -k_2 Q^2 \left[\frac{1}{2} [a_2^2 + b_2^2] + a_2 b_2 \sin 2Rt + \frac{1}{2} [a_2^2 - b_2^2] \cos 2Rt \right] \end{aligned}$$

$$\begin{aligned}
& -k_2 Q^2 \left[+ \frac{2a_2 S \bar{\xi}_2}{R^2} \cos R t + \frac{2b_2 S \bar{\xi}_2}{R^2} \sin R t \right] \\
& + k_1 Q^2 \left[\frac{1}{2} [a_1^2 + b_1^2] + a_1 b_1 \sin 2Q t + \frac{1}{2} [a_1^2 - b_1^2] \cos 2Q t \right] \\
& + k_1 Q^2 \left[+ \frac{2a_1 \bar{\xi}_1}{Q^2} \cos Q t + \frac{2b_1 \bar{\xi}_1}{Q^2} \sin Q t \right] + 2Q [a_1' \sin Q t - b_1' \cos Q t] \quad (36)
\end{aligned}$$

Now, to ensure a uniformly valid asymptotic solution in t , we equate to zero, in (36), the coefficients of $\cos Q t$ and $\sin Q t$ to get

$$b_1' + \alpha_1 b_1 = a_1 \varphi \quad (37a)$$

and

$$a_1' + \alpha_1 a_1 = -b_1 \varphi \quad (37b)$$

where,

$$\varphi = \frac{d(\)}{d\tau},$$

$$\varphi = \frac{1}{2Q} \left[1 + 2k_1 \bar{\xi}_1 \right]$$

Simplification of (37a, b) yield

$$b_1'' + \alpha_1 b_1' = -\varphi \left[b_1 \left[\varphi + \frac{\alpha_1^2}{\varphi} \right] + \frac{\alpha_1 b_1'}{\varphi} \right]$$

$$b_1'' + 2\alpha_1 b_1' + \varphi b_1 \left[\varphi + \frac{\alpha_1^2}{\varphi} \right] = 0$$

$$b_1(0) = 0; b_1'(0) = -\frac{\varphi \bar{\xi}_1}{Q^2} \quad (37c)$$

And

$$a_1'' + \alpha_1 a_1' = -\varphi \left[a_1 \left[\varphi + \frac{\alpha_1^2}{\varphi} \right] + \frac{\alpha_1 a_1'}{\varphi} \right]$$

$$176 \quad a_1'' + 2\alpha_1 a_1' + \alpha_1 \left[\varphi + \frac{\alpha_1^2}{\varphi} \right] = 0$$

$$177 \quad a_1(0) = -\frac{\bar{\xi}_1}{Q^2}; a_1'(0) = \frac{\alpha_1 \bar{\xi}_1}{Q^2} \quad (37d)$$

178 The remaining part of the equation in the substitution into (28) as obtained from (36) is

$$179 \quad \zeta_{,tt}^{(2)} + Q^2 \zeta^{(2)} = q_1 + k_1 Q^2 [p_0(\tau) \sin 2Qt + p_1(\tau) \cos 2Qt] \\ 180 \quad -k_2 Q^2 [p_2(\tau) \sin 2Rt + p_3(\tau) \cos 2Rt + p_4(\tau) \cos Rt + p_5(\tau) \sin Rt] \quad (38a)$$

$$181 \quad \zeta^{(2)}(0,0) = 0; \zeta_{,t}^{(2)}(0,0) + \zeta_{,\tau}^{(2)}(0,0) = 0 \quad (38b)$$

182 where,

$$183 \quad q_1 = \frac{\bar{\xi}_1}{Q^2} + k_1 Q^2 r_0(\tau) - k_2 Q^2 r_1(\tau); p_0(\tau) = a_1 b_1; p_1(\tau) = \frac{1}{2} [a_1^2 - b_1^2] \quad (38c)$$

$$184 \quad p_2(\tau) = a_2 b_2; p_3(\tau) = \frac{1}{2} [a_2^2 - b_2^2]; p_4(\tau) = \frac{2a_2 S \bar{\xi}_2}{R^2}; p_5(\tau) = \frac{2b_2 S \bar{\xi}_2}{R^2} \quad (38d)$$

$$185 \quad r_0(\tau) = \frac{1}{2} [a_2^2 + b_2^2]; r_1(\tau) = \frac{1}{2} [a_1^2 + b_1^2] \quad (38e)$$

$$186 \quad p_0(0) = 0; p_1(0) = \frac{\bar{\xi}_1^2}{2Q^4}; p_2(0) = 0; p_3(0) = \frac{S^2 \bar{\xi}_2^2}{2R^4} \quad (38f)$$

$$187 \quad p_4(0) = \frac{2S^2 \bar{\xi}_2^2}{R^4}; p_5(0) = 0; r_0(0) = \frac{\bar{\xi}_1^2}{2Q^4}; r_1(0) = \frac{S^2 \bar{\xi}_2^2}{2R^4}; \quad (38g)$$

188 The solution of (38a), using (38b) is

$$189 \quad \zeta^{(2)}(t, \tau) = a_3(\tau) \cos Qt + b_3(\tau) \sin Qt + \frac{q_1}{Q^2} - \frac{k_1}{3} [p_6(\tau) \sin 2Qt + p_7(\tau) \cos 2Qt]$$

190

$$191 \quad -k_2 Q^2 [p_8(\tau) \sin 2Rt + p_9(\tau) \cos 2Rt + p_{10}(\tau) \cos Rt + p_{11}(\tau) \sin Rt] \quad (39a)$$

$$192 \quad a_3(0) = \bar{\xi}_1 l_0 + k_1 \bar{\xi}_1^2 l_1 + k_2 \bar{\xi}_2^2 \left[\frac{S}{R^2} \right]^2 l_2; b_3(0) = -\frac{\alpha_1 \bar{\xi}_1}{Q^3} \quad (39b)$$

193 where

$$l_0 = -\frac{1}{Q^4}; l_1 = -\frac{1}{2Q^2} + \frac{1}{6Q^4}; l_2 = \frac{1}{2} + \frac{1}{2[Q^2 - 4R^2]} - \frac{2}{R^2[Q^2 - 4R^2]} \quad (39c)$$

$$p_6(\tau) = a_1 b_1; p_7(\tau) = a_2 b_2; p_8(\tau) = \frac{p_2(\tau)}{Q^2 - 4R^2}; p_9(\tau) = \frac{p_3(\tau)}{Q^2 - 4R^2} \quad (39d)$$

$$p_{10}(\tau) = \frac{p_4(\tau)}{Q^2 - R^2}; p_{11}(\tau) = \frac{p_5(\tau)}{Q^2 - R^2}; p_{11}(0) = 0 \quad (39e)$$

$$p_6(0) = 0; p_7(0) = \frac{\bar{\xi}_1^2}{2Q^4}; p_8(0) = 0; p_9(0) = \frac{S^2 \bar{\xi}_2^2}{2R^4[Q^2 - 4R^2]}; p_{10}(0) = \frac{2S^2 \bar{\xi}_2^2}{R^4[Q^2 - R^2]} \quad (39f)$$

Substituting using (34a) and (35a) into (30) we get

$$\begin{aligned} \eta_{,tt}^{(2)} + R^2 \eta^{(2)} = & -2\alpha_2 [-Ra_2 \sin Rt + Rb_2 \cos Rt] - 2R [-a_2' \sin Rt + b_2' \cos Rt] \\ & + S \left[a_2 \cos Rt + b_2 \sin Rt + \frac{S \bar{\xi}_2}{R^2} \right] \\ & - \frac{R^2}{2} \left[\frac{2S \bar{\xi}_1 \bar{\xi}_2}{[QR]^2} + \frac{2a_2 \bar{\xi}_1}{Q^2} \cos Rt + \frac{2b_2 \bar{\xi}_1}{Q^2} \sin Rt + \frac{2a_1 S \bar{\xi}_2}{R^2} \cos Qt + \frac{2b_1 S \bar{\xi}_2}{R^2} \sin Qt \right. \\ & + [a_1 a_2 - b_1 b_2] \cos[Q - R]t + [a_1 b_2 + b_1 a_2] \sin[Q + R]t \\ & \left. + [a_1 a_2 + b_1 b_2] \cos[Q - R]t + [b_1 a_2 - a_1 b_2] \sin[Q - R]t \right] \end{aligned} \quad (40)$$

Now, to ensure a uniformly valid asymptotic solution in t , we equate the coefficients of $\cos Rt$ and $\sin Rt$ to zero. This will ensure a finite at infinite time, i.e. as t tends to infinity, such terms also tends to infinity thereby making the solution not to be bounded, hence non-uniform. Such terms are called secular terms and our aim is to get rid of them.

$$b_2' + \alpha_2 b_2 = a_2 \Phi \quad (41a)$$

and

$$a_2' + \alpha_2 a_2 = -b_2 \Phi \quad (41b)$$

where,

$$\Phi = \frac{1}{2R} \left[S - \frac{R^2 \bar{\xi}_1}{Q^2} \right].$$

210 Simplification of (41a, b) yield

$$211 \quad b_2'' + \alpha_2 b_2' = -\Phi[\Phi b_2 + \alpha_2 a_2]$$

$$212 \quad b_2'' + \alpha_2 b_2' = -\Phi\left[\Phi b_2 + \frac{\alpha_2}{\Phi}[b_2' + \alpha_2 b_2]\right]$$

$$213 \quad b_2'' + 2\alpha_2 b_2' + b_2[\Phi^2 + \alpha_2^2] = 0$$

$$214 \quad b_2(0) = 0; b_2'(0) = -\frac{\Phi S \bar{\xi}_2}{R^2} \quad (41c)$$

215 And

$$216 \quad a_2'' + \alpha_2 a_2' = -\Phi[\Phi a_2 - \alpha_2 b_2]$$

$$217 \quad a_2'' + \alpha_2 a_2' = -\Phi\left[\Phi a_2 + \frac{\alpha_2}{\Phi}[a_2' + \alpha_2 a_2]\right]$$

$$218 \quad a_2'' + 2\alpha_2 a_2' + a_2[\Phi^2 + \alpha_2^2] = 0$$

$$219 \quad a_2(0) = -\frac{S \bar{\xi}_2}{R^2}; a_2'(0) = -\frac{\alpha_2 S \bar{\xi}_2}{R^2} \quad (41d)$$

220 The remaining part of the equation in the substitution into (30) as obtained from (40) is

221

$$222 \quad \eta_{,tt}^{(2)} + R^2 \eta^{(2)} = q_2 - \frac{R^2}{2} \left[p_{12}(\tau) \cos Qt + p_{13}(\tau) \sin Qt + p_{14}(\tau) \cos[Q + R]t \right. \\ \left. + p_{15}(\tau) \sin[Q + R]t + p_{16}(\tau) \cos[Q - R]t + p_{17}(\tau) \sin[Q - R]t \right] \\ 223 \quad \eta^{(2)}(0,0) = 0; \eta_{,t}^{(2)}(0,0) + \eta_{,\tau}^{(1)}(0,0) = 0 \quad (42b)$$

224 where,

$$225 \quad q_2 = \frac{S^2 \bar{\xi}_2}{R^2} - \frac{S \bar{\xi}_1 \bar{\xi}_2}{Q^2}; p_{12}(\tau) = \frac{2a_1 S \bar{\xi}_2}{R^2}; p_{13}(\tau) = \frac{2b_1 S \bar{\xi}_2}{R^2}; p_{14}(\tau) = a_1 a_2 - b_1 b_2 \quad (42c)$$

226

$$227 \quad p_{15}(\tau) = b_1 b_2 + b_1 a_2; p_{16}(\tau) = a_1 a_2 + b_1 b_2; p_{17}(\tau) = b_1 a_2 - a_1 b_1 \quad (42d)$$

$$228 \quad p_{12}(0) = \frac{2S \bar{\xi}_1 \bar{\xi}_2}{Q^2 R^2}; p_{13}(0) = 0; p_{14}(0) = \frac{S \bar{\xi}_1 \bar{\xi}_2}{Q^2 R^2}; p_{15}(0) = 0; \quad (42e)$$

$$229 \quad p_{16}(0) = \frac{S \bar{\xi}_1 \bar{\xi}_2}{Q^2 R^2}; p_{17}(0) = 0 \quad (42f)$$

230 The solution of (42a) using (42b) is

$$231 \quad \eta^{(2)}(t, \tau) = a_4(\tau) \cos Rt + b_4(\tau) \sin Rt + \frac{q_2}{R^2} - \frac{1}{2} \left[\begin{aligned} & p_{18}(\tau) \cos Qt + p_{19}(\tau) \sin Qt + \\ & p_{20}(\tau) \cos[Q+R]t + p_{21}(\tau) \sin[Q+R]t + \\ & p_{22}(\tau) \cos[Q-R]t + p_{23}(\tau) \sin[Q-R]t \end{aligned} \right] \quad (43a)$$

$$232 \quad a_4(0) = S^2 \bar{\xi}_2 l_3 + R^2 S \bar{\xi}_1 \bar{\xi}_2 l_4; b_4(0) = -\frac{\alpha_2 S \bar{\xi}_2}{R^3} \quad (43b)$$

233 where,

$$234 \quad l_4 = \left[\frac{1}{[R^2 Q]^2} + \frac{1}{2} \left[\frac{-2}{[RQ]^2 [R^2 - Q^2]} - \frac{1}{Q[RQ]^2 [2R+Q]} + \frac{1}{Q[RQ]^2 [2R-Q]} \right] \right] \quad (43c)$$

235

$$236 \quad l_3 = -\frac{1}{R^4}; p_{18}(\tau) = \frac{p_{12}(\tau)}{R^2 - Q^2}; p_{19}(\tau) = \frac{p_{13}(\tau)}{R^2 - Q^2}; p_{20}(\tau) = \frac{p_{14}(\tau)}{Q[2R+Q]} \quad (43d)$$

237

$$238 \quad p_{21}(\tau) = \frac{p_{15}(\tau)}{Q[2R+Q]}; p_{22}(\tau) = \frac{p_{16}(\tau)}{Q[2R-Q]}; p_{23}(\tau) = \frac{p_{17}(\tau)}{Q[2R-Q]} \quad (43e)$$

$$239 \quad p_{18}(0) = \frac{2S \bar{\xi}_1 \bar{\xi}_2}{Q^2 R^2 [R^2 - Q^2]}; p_{19}(0) = 0; p_{20}(0) = \frac{S \bar{\xi}_1 \bar{\xi}_2}{Q^3 R^2 [2R+Q]} \quad (43f)$$

240

$$241 \quad p_{21}(0) = 0; p_{22}(0) = \frac{S \bar{\xi}_1 \bar{\xi}_2}{Q^2 R^2 [2R-Q]}; p_{23}(0) = 0 \quad (43g)$$

242

243 **Next, using (34a), (39a) and (35a), (43a) we deduce the displacements as**

$$244 \quad \xi_1(t) = \varsigma^{(1)}(t, \tau) \varepsilon + \varsigma^{(2)}(t, \tau) \varepsilon^2 + \dots \quad (44a)$$

245 and

$$246 \quad \xi_2(t) = \eta^{(1)}(t, \tau) \varepsilon + \eta^{(2)}(t, \tau) \varepsilon^2 + \dots \quad (44b)$$

247 We seek the maximum displacement for both $\xi_1(t)$ and $\xi_2(t)$. To achieve this, we shall first

248 determine the critical values of t and τ for each of $\xi_1(t)$ and $\xi_2(t)$ at their maximum values.

249 The condition for the maximum displacements of $\xi_1(t)$ and $\xi_2(t)$ is obtain from (24b).hence

$$250 \quad \xi_{1,t} + \varepsilon \xi_{1,\tau}, \quad (45a)$$

$$\xi_{2,t} + \varepsilon \xi_{2,\tau}, \quad (45b)$$

We know from (44a, b) that

$$\xi_1(t) = \varsigma^{(1)}(t, \tau) \varepsilon + \varsigma^{(2)}(t, \tau) \varepsilon^2 + \dots \quad (46a)$$

$$\xi_2(t) = \eta^{(1)}(t, \tau) \varepsilon + \eta^{(2)}(t, \tau) \varepsilon^2 + \dots \quad (46b)$$

On applying (45a, b) to (46a, b), we get

$$\begin{aligned} \varsigma_{,t} + \varepsilon \varsigma_{,\tau} &= [\varsigma_{,t}^{(1)}(t_a, \tau_a) \varepsilon + \varsigma_{,t}^{(2)}(t_a, \tau_a) \varepsilon^2 + \dots] \\ &+ \varepsilon [\varsigma_{,\tau}^{(1)}(t_a, \tau_a) \varepsilon + \varsigma_{,\tau}^{(2)}(t_a, \tau_a) \varepsilon^2 + \dots] = 0 \end{aligned} \quad (47a)$$

And

$$\begin{aligned} \eta_{,t} + \varepsilon \eta_{,\tau} &= [\eta_{,t}^{(1)}(T_c, \tau_c) \varepsilon + \eta_{,t}^{(2)}(T_c, \tau_c) \varepsilon^2 + \dots] \\ &+ \varepsilon [\eta_{,\tau}^{(1)}(T_c, \tau_c) \varepsilon + \eta_{,\tau}^{(2)}(T_c, \tau_c) \varepsilon^2 + \dots] = 0 \end{aligned} \quad (47b)$$

where, (t_a, τ_a) and (T_c, τ_c) are the values of t and τ at the maximum displacement of

$\varsigma(t, \tau)$ and $\eta(t, \tau)$ respectively.

We now expand (47a, b) in a Taylor series about $t_a = t_0, \tau_a = 0$ and $T_c = T_0, \tau_c = 0$, and

thereafter equate to zero the terms of the same orders of ε to get

$$\varsigma_{,t}^{(1)}(t_0, 0) = 0 \quad (48a)$$

$$t_1 \varsigma_{,tt}^{(1)}(t_0, 0) + t_0 \varsigma_{,t\tau}^{(1)}(t_0, 0) + \varsigma_{,t}^{(2)}(t_0, 0) + \varsigma_{,\tau}^{(1)}(t_0, 0) = 0 \quad (48b)$$

And

$$\eta_{,t}^{(1)}(T_0, 0) = 0 \quad (49a)$$

$$T_1 \eta_{,tt}^{(1)}(T_0, 0) + T_0 \eta_{,t\tau}^{(1)}(T_0, 0) + \eta_{,t}^{(2)}(T_0, 0) + \eta_{,\tau}^{(1)}(T_0, 0) = 0 \quad (49b)$$

Substituting for $\varsigma_{,t}^{(1)}$ from (34a) in (48a) and simplifying we get

$$\sin Qt_0 = 0 \quad (50a)$$

A further simplification of (50a) gives

$$t_0 = \frac{\pi}{Q} \quad (50b)$$

A similar solution for (49a) is

$$T_0 = \frac{\pi}{R} \quad (50c)$$

Next, we deduce from (48b) that

$$t_1 = -\frac{1}{\varsigma_{,tt}^{(1)}(t_0,0)} \left[t_0 \varsigma_{,t\tau}^{(1)}(t_0,0) + \varsigma_{,t}^{(2)}(t_0,0) + \varsigma_{,\tau}^{(1)}(t_0,0) \right] \quad (51a)$$

Simplification of the following terms are however necessary in this analysis,

$$\varsigma_{,t}^{(2)}(t_0,0) = \alpha_1 \bar{\xi}_1 l_5 + k_1 \bar{\xi}_1 l_6 - k_2 S^2 \bar{\xi}_2 l_7; \varsigma_{,t}^{(1)}(t_0,0) = \bar{\xi}_1 l_8 \quad (51b)$$

$$\varsigma_{,\tau}^{(1)}(t_0,0) = \bar{\xi}_1 l_9; \varsigma_{,tt}^{(1)}(t_0,0) = \bar{\xi}_1; \varsigma^{(1)}(t_0,0) = 2 \bar{\xi}_1 l_{10} \quad (51c)$$

$$\varsigma^{(2)}(t_0,0) = 2 \bar{\xi}_1 l_{11} + k_1 \bar{\xi}_1 l_{12} + k_2 \bar{\xi}_2 \left[\frac{S}{R^2} \right]^2 l_{13}; \varsigma_{,\tau}^{(1)}(t_0,0) = 0; \quad (51d)$$

where

$$l_5 = \frac{1}{Q^2}; l_6 = \frac{\sin 2Qt_0}{3Q^2}; l_7 = \frac{Q^2 \sin 2Rt_0}{R^3 [Q^2 - 4R^2]} \quad (51e)$$

$$l_8 = \frac{\varphi}{Q}; l_9 = -\frac{\alpha_1}{Q^2}; l_{10} = \frac{1}{Q^2}; l_{11} = \frac{1}{Q^4}; l_{12} = \frac{1}{Q^2} - \frac{1}{3Q^4} \quad (51f)$$

$$l_{13} = \left[-1 - \frac{1}{2[Q^2 - 4R^2]} + \frac{1}{R^2[Q^2 - 4R^2]} - Q^2 \left[\frac{1}{Q^2 - 4R^2} + \frac{2}{Q^2 - R^2} \right] \right] \quad (51g)$$

On substituting (51, b-d) on (51a), we have

$$t_1 = \alpha_1 l_5 + k_1 \bar{\xi}_1 l_6 - k_2 S \bar{\xi}_2 l_7 + t_0 l_8 + l_9 \quad (52)$$

Similarly, deducing from (49b) yields

$$T_1 = -\frac{1}{\eta_{,tt}^{(1)}(T_0,0)} \left[T_0 \eta_{,t\tau}^{(1)}(T_0,0) + \eta_{,t}^{(2)}(T_0,0) + \eta_{,\tau}^{(1)}(T_0,0) \right] \quad (53a)$$

We however note the following simplifications

$$\eta_{,t}^{(2)}(T_0,0) = \alpha_2 S \bar{\xi}_2 l_{14} + S^2 \bar{\xi}_2 l_{15} + S \bar{\xi}_1 \bar{\xi}_2 l_{16}; \eta_{,t\tau}^{(1)}(T_0,0) = S \bar{\xi}_2 l_{17} \quad (53b)$$

$$\eta_{,\tau}^{(1)}(T_0,0) = S^2 \bar{\xi}_2 l_{18}; \eta_{,tt}^{(1)}(T_0,0) = -S \bar{\xi}_2; \eta^{(1)}(T_0,0) = 2S \bar{\xi}_2 l_{20} \quad (53c)$$

$$\eta^{(2)}(T_0, 0) = S^2 \bar{\xi}_2 l_{21} + R^2 S \bar{\xi}_1 \bar{\xi}_2 l_{19}; \eta^{(1)}_{,t}(T_0, 0) = 0; \quad (53d)$$

where

$$l_{14} = -\frac{\cos RT_0}{R^2}; l_{15} = -Rl_3 \sin RT_0 \quad (53e)$$

$$l_{16} = -R^3 S l_4 \sin RT_0 - \frac{R^2}{2} \left[\frac{2 \sin QT_0}{QR^2[R^2 - Q^2]} - \frac{\cos QT_0}{[RQ]^2[2R + Q]} - \frac{[Q + R] \sin[Q + R]T_0}{Q[RQ]^2[2R + Q]} - \frac{[Q - R] \sin[Q - R]T_0}{Q[RQ]^2[2R - Q]} \right] \quad (53f)$$

$$l_{17} = \frac{\Phi}{R}; l_{18} = -\frac{\alpha_2}{R^2}; l_{20} = \frac{1}{R^2}; l_{21} = \frac{1}{R^4}; l_{21} = \frac{1}{R^4} \quad (53g)$$

$$l_{19} = \left[-l_4 + \frac{1}{2} \left[\frac{2 \cos QT_0}{Q^2 R^2 [R^2 - Q^2]} + \frac{\cos[Q + R]T_0}{Q[RQ]^2[2R + Q]} + \frac{\cos[Q - R]T_0}{Q[RQ]^2[2R - Q]} \right] \right] \quad (53h)$$

On substituting (53, b-d) on (53a), we have

$$T_1 = \alpha_2 l_{14} + \bar{\xi}_1 l_{16} + T_0 l_{17} + l_{18} \quad (54)$$

We, now, determine the maximum values of $\zeta(t)$ and $\eta(t)$ say ζ_a and η_c respectively by

evaluating (46 a, b) at the critical values namely $t = t_a, \tau = \tau_a$ and $T = T_c, \tau = \tau_c$.

$$\zeta_a = \zeta^{(1)}(t_a, \tau_a) \mathcal{E} + \zeta^{(2)}(t_a, \tau_a) \mathcal{E}^2 + \dots \quad (55a)$$

$$\eta_c = \eta^{(1)}(T_c, \tau_c) \mathcal{E} + \eta^{(2)}(T_c, \tau_c) \mathcal{E}^2 + \dots \quad (55b)$$

Expanding (55 a) in Taylor series using,

$$t_a = t_0 + \mathcal{E} t_1 + \mathcal{E}^2 t_2 + \dots; \tau_a = \mathcal{E} t_a = \mathcal{E} [t_0 + \mathcal{E} t_1 + \mathcal{E}^2 t_2 + \dots] \quad (56a)$$

we have

$$\begin{aligned} \zeta_a &= \mathcal{E} [\zeta^{(1)}(t_0, 0) + \zeta^{(1)}_{,t}(t_0, 0) [\mathcal{E} t_1 + \mathcal{E}^2 t_2 + \dots] + \zeta^{(1)}_{,\tau}(t_0, 0) \mathcal{E} [t_0 + t_1 \mathcal{E} + \dots]] \\ &+ \zeta^{(2)}(t_0, 0) \mathcal{E}^2 + \dots \end{aligned} \quad (56b)$$

Regrouping the terms in orders of \mathcal{E} yields

$$\zeta_a = \mathcal{E} \zeta^{(1)}(t_0, 0) + \mathcal{E}^2 [t_1 \zeta^{(1)}_{,t}(t_0, 0) + t_0 \zeta^{(1)}_{,\tau}(t_0, 0) + \zeta^{(2)}(t_0, 0)] + \dots \quad (56c)$$

On substituting the terms in (56c) from (51, b-d), we have

$$\zeta_a = 2 \bar{\xi}_1 l_{10} \mathcal{E} + \left[t_0 \bar{\xi}_1 l_9 + 2 \bar{\xi}_1 l_{11} + k_1 \bar{\xi}_1 l_{12} + k_2 \bar{\xi}_2 \left[\frac{S}{R^2} \right]^2 l_{13} \right] \mathcal{E}^2 + \dots \quad (57)$$

319 Similarly, expanding (55 b) in Taylor series using,

$$320 \quad T_c = T_0 + \varepsilon T_1 + \varepsilon^2 T_2 + \dots; \tau_c = \varepsilon T_c = \varepsilon [T_0 + \varepsilon T_1 + \varepsilon^2 T_2 + \dots] \quad (58a)$$

321 we have

$$322 \quad \eta_c = \varepsilon [\eta^{(1)}(T_0, 0) + \eta_{,t}^{(1)}(T_0, 0) [\varepsilon T_1 + \varepsilon^2 T_2 + \dots] + \eta_{,t}^{(1)}(T_0, 0) \varepsilon [T_0 + \varepsilon T_1 + \dots]] \\ 323 \quad + \eta^{(2)}(T_0, 0) \varepsilon^2 + \dots \quad (58b)$$

324 Regrouping the terms in orders of ε yields

$$325 \quad \eta_c = \varepsilon \eta^{(1)}(T_0, 0) + \varepsilon^2 [T_1 \eta_{,t}^{(1)}(T_0, 0) + T_0 \eta_{,t}^{(1)}(T_0, 0) + \eta^{(2)}(T_0, 0)] + \dots \quad (58c)$$

326 On substituting the terms in (58c) from (53, b-d), we have

$$327 \quad \eta_c = 2S \bar{\xi}_2 l_{20} \varepsilon + \left[T_0 S^2 \bar{\xi}_2 l_{18} + S^2 \bar{\xi}_2 l_{21} + R^2 S \bar{\xi}_1 \bar{\xi}_2 l_{19} \right] \varepsilon^2 + \dots \quad (59)$$

328 The net maximum displacement ξ_m is

$$329 \quad \xi_m = \varsigma_a + \eta_c = \varsigma(t_a, \tau_a) + \eta(T_c, \tau_c) \quad (60)$$

330 Substituting for terms in (60) from (57) and (59) we get

$$331 \quad \xi_m = C_1 \varepsilon + C_2 \varepsilon^2 + \dots \quad (61a)$$

332 where

$$333 \quad C_1 = l_{22}; C_2 = \bar{\xi}_1 l_{23} + S^2 \bar{\xi}_2 l_{24} + k_1 \bar{\xi}_1 l_{12} + k_2 \bar{\xi}_2 \left[\frac{S}{R^2} \right]^2 l_{13} + R^2 \bar{\xi}_1 \bar{\xi}_2 S l_{19} \quad (61b)$$

$$334 \quad l_{22} = 2 \bar{\xi}_1 l_{10} + 2S \bar{\xi}_2 l_{20}; l_{23} = 2l_{11} + t_0 l_9; l_{24} = l_{21} + T_0 l_{18} \quad (61c)$$

335 As noted by [1–3] and [21], the condition for dynamic buckling is

$$336 \quad \frac{d\lambda}{d\xi_m} = 0 \quad (62)$$

337 As in [23–24], applying the method of reversal of series of (61a), we get

$$338 \quad \varepsilon = d_1 \xi_m + d_2 \xi_m^2 + \dots \quad (63)$$

339 Substituting for ξ_m from (61a) in (63) and equating powers of orders of ε , we get

$$340 \quad d_1 = \frac{1}{C_1}, d_2 = -\frac{C_2}{C_1^3} \quad (64)$$

341 The maximization in (62) is better done from (63), thus implementing (62) using (63) we
342 have

$$\xi_m(\lambda_D) = \frac{C_1^2}{2C_2} \quad (65)$$

where, $\xi_m(\lambda_D)$ is the value of the net displacement at buckling. In determining the dynamic buckling load, we evaluate (63) at

$$\lambda = \lambda_D$$

to yield

$$\varepsilon = \xi_m(\lambda_D)[d_1 + d_2 \xi_m]_{(\lambda=\lambda_D)} \quad (66)$$

On substituting for terms d_1 and d_2 from (64) and $\xi_m(\lambda_D)$ from (65) in (66) and simplify to get

$$\varepsilon \lambda_D = \frac{C_1}{4C_2} \quad (67)$$

The expansion of (67) gives [using (61b, c)]

$$\lambda_D = \frac{1}{4} \left[\frac{\omega_0}{\omega_1} \right]^2 \left[2 \bar{\xi}_1 l_{10} + 2S \bar{\xi}_2 l_{20} \right] \left[\begin{array}{c} \bar{\xi}_1 l_{23} + S^2 \bar{\xi}_2 l_{24} + k_1 \bar{\xi}_1^2 l_{12} + k_2 \bar{\xi}_2^2 \left[\frac{S}{R^2} \right]^2 l_{13} \\ + R^2 \bar{\xi}_1 \bar{\xi}_2 S l_{19} \end{array} \right]^{-1} \quad (68)$$

Here, (68) gives the formula for evaluating the dynamic buckling load λ_D , and is valid for

$$R \neq (1, 2, Q, 2Q, 1 - Q, 1 + Q) \text{ and } Q \neq (R, 2R, 1 - R, 1 + R, 0, 2R - 1)$$

355

356 4. ANALYSIS OF RESULT.

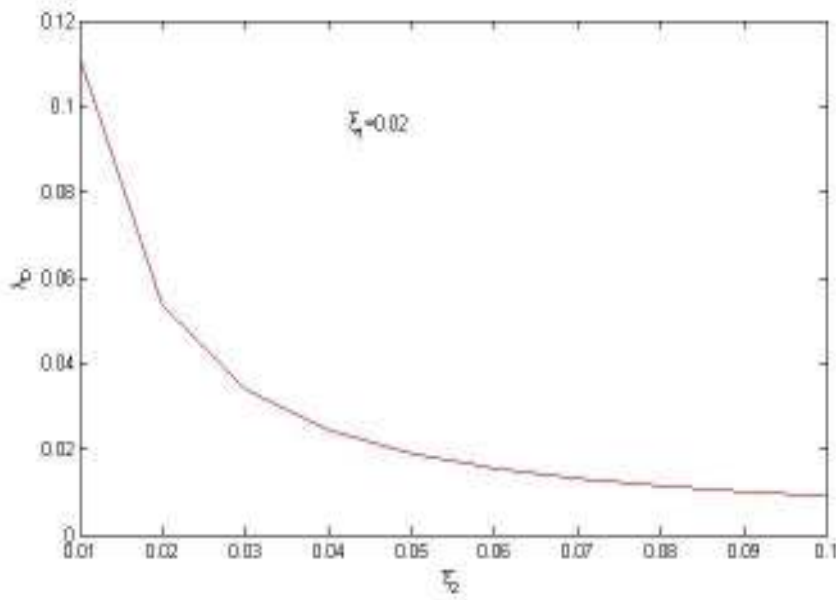
357

358 The above results indicate that dynamic buckling load increases if the structure is less
359 imperfect. The results also show that dynamic buckling load increases with increased
360 damping. In addition, the results confirm that the only condition in which the effect of the
361 coupling between the buckling modes is felt is if none of the imperfection parameters in the
362 shape of the mode coupling is neglected. Once an imperfection is neglected the coupling
363 effect of the mode that is in the shape of the neglected imperfection, with any other mode is
364 neglected. For a graphical view of this phenomenon, we use the following values. $k_1=0.2$,

365 $k_2=0.3$, $\bar{\xi}_1=0.01$, $\bar{\xi}_2=0.03$, $\alpha_1=0.01$ and $\alpha_2=0.03$. By varying $\bar{\xi}_1$ and α_1 while keeping

366 $\bar{\xi}_2$ constant at 0.03 and $\alpha_2 = 0$, the corresponding values of λ_D were computed from (68).

367 The plots of dynamic buckling load against the imperfection parameter and light viscous
 368 damping of the discretized spherical cap are shown in figures 1 and 2 below.



369

370

371 Figure 1: Dynamic buckling load of a spherical cap against the imperfection parameter

372 $\bar{\xi}_2 (\bar{\xi}_1 = 0.02)$

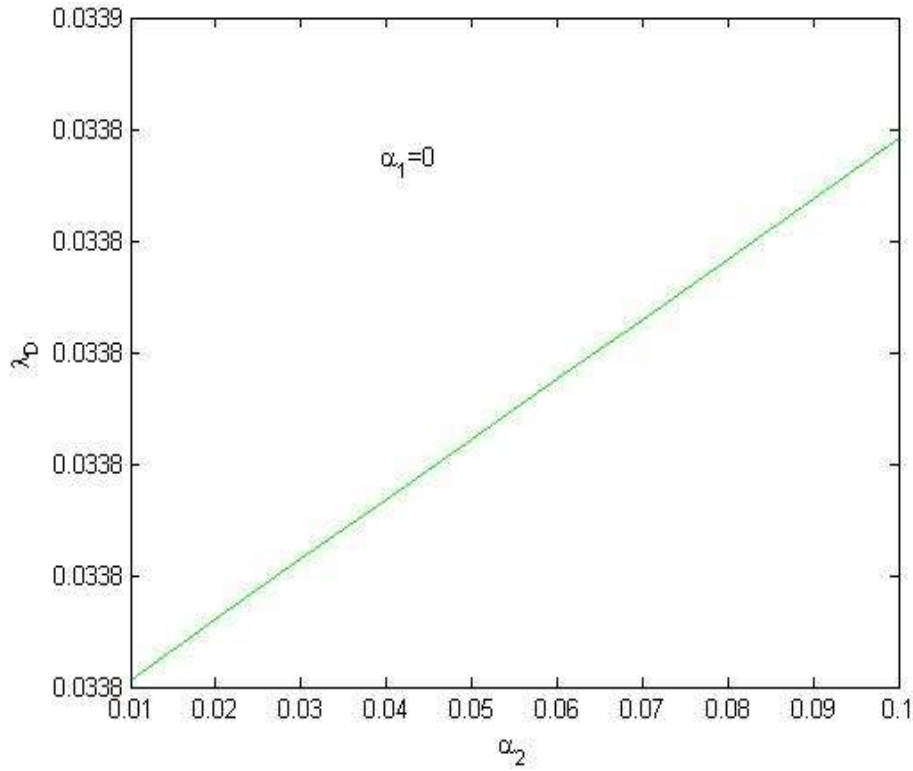


Figure 2: Dynamic buckling load of a spherical cap against the light viscous damping α_2 ($\alpha_1 = 0$)

We note that the results display all the imperfection parameters stated in problems (3)-(5). This is unlike Danielson's problem in which the axisymmetric imperfection was neglected for easy solution. In fact, the method is such that we can adequately account for all modal imperfections allowed in the formulation. The contributions of the quadratic terms $k_1 \bar{\xi}_1^2, k_2 \bar{\xi}_2^2$ and the coupling term $\bar{\xi}_1 \bar{\xi}_2$ are respectively given in the denominator of (68) by

$$k_1 \bar{\xi}_1^2 l_{11}, k_2 \bar{\xi}_2^2 \left[\frac{S}{R^2} \right] l_{13} \text{ and } R^2 \bar{\xi}_1 \bar{\xi}_2 S l_{19}. \text{ Thus if we assume that the axisymmetric}$$

imperfections are zero then $\bar{\xi}_1 = 0$, and the dynamic buckling load λ_D responsible for the buckling in this case is obtained from (68) as

$$\lambda_D = \frac{1}{4} \left[\frac{\omega_0}{\omega_1} \right]^2 \left[\left[2S \bar{\xi}_2 l_{20} \right] \left[S^2 \bar{\xi}_2 l_{24} + k_2 \bar{\xi}_2^2 \left[\frac{S}{R^2} \right]^2 l_{13} \right]^{-1} \right] \quad (69)$$

We note from (69), that, the effect of the coupling terms $\xi_1 \xi_2$, $\xi_1 \xi_0$ and $k_1 \xi_1^2$ are zeros. The effect of the quadratic term $k_2 \xi_2^2$ is non-zero and it is this term that dominates the buckling process. Neglecting $\bar{\xi}_1$ is sufficient to completely nullify the effect of ξ_1^2 where the converse is not necessarily the case. However, if the non-axymmetric imperfections are neglected then $\bar{\xi}_2 = 0$, and the dynamic buckling load λ_D following (68) become

$$\lambda_D = \frac{1}{4} \left[\frac{\omega_0}{\omega_1} \right]^2 \left[\left[2 \bar{\xi}_1 l_{10} \right] \left[\left[\bar{\xi}_1 l_{23} + k_1 \bar{\xi}_1^2 l_{12} \right]^{-1} \right] \right] \quad (70)$$

We deduce from (70), that, the effect of the coupling terms $\xi_1 \xi_2$, $\xi_2 \xi_0$ and $k_2 \xi_2^2$ are again zeros. The effect of the quadratic term $k_1 \xi_1^2$ is non-zero and this singular term is the only non-linear term that influences the buckling process. Neglecting $\bar{\xi}_2$ is sufficient to completely nullify the effect of ξ_2^2 where the converse is not necessarily the case.

5. CONCLUSION.

From the above discussions, we note that while neglecting the imperfection parameters $\bar{\xi}_1$ and $\bar{\xi}_2$ automatically implies, among other things, neglecting the effects of the non-linear terms $k_1 \xi_1^2$ and $k_2 \xi_2^2$ respectively. Also, we observe that the only condition under which the effect of the coupling term $\xi_1 \xi_2$ would be felt, is when the imperfection parameters $\bar{\xi}_1$ and $\bar{\xi}_2$ are not equal to zero. Moreover, our results confirm those obtained by [21]. Finally, we notice that we can determine the value of the dynamic buckling load λ_D for whatever number of modal imperfections.

REFERENCES

- [1] Hutchinson, J.W and Budiansky, B., Dynamic buckling estimates, AIAA J. 1966; 4: 52-530.

- 409 [2] Budiansky, B. and Hutchinson, J.W., *Dynamic buckling of imperfection- sensitive*
410 *structure, proceedings of the XIth Inter. Congr. Applied Mech. Springer-Verlag, Berlin. 1966*
- 411 [3] Budiansky, B., *Dynamic buckling of elastic structures; Criteria and estimates, in, Dynamic*
412 *stability of structures. Pergamons, New York. 1966*
- 413 [4] Ohsaki, M., *Imperfection sensitivity of optimal, symmetric braced frames against*
414 *buckling. Int. J. Non-Linear Mech., 2003; 3(7):1103-1117.*
- 415 [5] Dumir, P.C., Dube, G.P. and Mullick, A. *Axisymmetric static and dynamic buckling of*
416 *laminated thick truncated conical cap." Int. J. Non-Linear Mech., 2003; 38(6):903-910.*
- 417 [6] Kardomateas, G.A., Simitses, G.J., Shem, L. and Li, R.,
418 *Buckling of sandwich wide columns, Int. J. Non-Linear Mech; 2002; 37(7): 1239-1247.*
- 419 [7] Anwen, W. and Wenying, *Characteristic-value analysis for plastic dynamic buckling of*
420 *columns under elastoplastic compression waves, Int. J. Non-Linear Mech. 2003; 38(5): 615-*
421 *628.*
- 422 [8] Rafteyiannis, I.G. and Kounadis, A., *Interaction step loading, Int. J. Non-Linear Mech.*
423 *2000; 35(3): 531-542.*
- 424 [9] Wei, Z.G., Yu, J.L. and Batra, R.C., *Dynamic buckling of thin cylindrical shells under axial*
425 *impact, Int. J. Impact Engng., 2005; 32:572-592.*
- 426 [10] Batra, R.C. and Wei, Z.G. (2005), *Dynamic buckling of thin thermoviscoplastic*
427 *rectangular plate, J. of Thin-Walled Structures, 2005; 43: 273-290.*
- 428 [11] Zhang, T., Liu, T.G., Zhao, Y. and Luo J.Z., *Nonlinear dynamic buckling of stiffened*
429 *plates under in-plane impact load. J. of Zhejiang University of Science, 2004; 5 (5): 609*
- 430 [12] Danielson, D., *Dynamic buckling loads of imperfection sensitive structures from*
431 *perturbation procedures, AIAA J. 1969; 7:1506.*
- 432 [13] Aksogan O. and Sofiyev, A.V., *Dynamic buckling of cylindrical shells with variable*
433 *thickness subjected to a time-dependent external pressure varying as a power function of*
434 *time, J. of Sound and vibration, 2002; 254(4): 693-703.*
- 435 [14] Schenk, C.A and Schueller, G.I., *Buckling of cylindrical shells with random*
436 *imperfections, Int. J. Non-Linear mech. 2003; 38: 1119-1132.*

- 437 [15] Wang, A. and Tian, W. Twin characteristics- parameter solution under elastic
438 compression waves. Int J. Solids Struct. 2002; 39, 861-877.
- 439 [16] Wang, A. and Tian, W. Twin characteristic parameter solutions of axisymmetric dynamic
440 plastic buckling for cylindrical shells under axial compression waves. Int. J. Solids
441 Struct. 2003; 40, 3157-3175
- 442 [17] Ette, A.M. On a two-small-parameter dynamic buckling of a lightly damped spherical cap
443 trapped by a step load. J. of Nigerian Math. Society, 2007; 23, 7-26.
- 444 [18] Ette, A.M. On a two-small-parameter dynamic stability of a lightly damped spherical
445 shell pressurized by a harmonic excitation. J. of Nigerian Assoc. of Math. Physics, 2007; 11,
446 333-362
- 447 [19] Ette, A.M. On the buckling of lightly damped cylindrical shells modulated by a periodic
448 load. J. of Nigerian Assoc. Math. Physics, 2006; 10, 327-344
- 449 [20] Ette, A.M. Perturbation technique on the dynamic stability of a damped cylindrical shell
450 axially stressed by an impulse. J. of Nigerian Assoc. Math. Physics, 2008; 12, 103-120.
- 451 [21] Ette, A.M. Dynamic buckling of imperfect spherical shell under an axial, Int. J. Non-Linear
452 Mech. 1997; 32(1) 201-209
- 453 [22] Ette, A.M. Perturbation approach on the dynamic buckling of a lightly damped
454 cylindrical shells modulated by a periodic load. J of Nigeria Math. Society, 2009; 28, 97-135.
- 455 [23] Amazigo, J.C. Buckling of stochastically imperfect columns of nonlinear elastic
456 foundations. Quart. App. Math. 1973; 31, 403.
- 457 [24] Amazigo, J.C. Dynamic buckling of structures with random imperfections. Stochastic
458 problem in Mechanics. Ed. H. Leipolz, University of Waterloo press, 1974; 243-254.
459